

Supplementary Material

S1 – Aggregation of Small Reservoirs into “Ponds” in SWAT Model

In the modeling of High-density Reservoir Network (HdRN), multiple small reservoirs can share the same sub-basin. In this case, there are two possible arrangements to be evaluated in the arrangement of small reservoirs in the sub-basins: the cascade arrangement and the parallel arrangement. In the cascade arrangement two or more reservoirs are located one behind the other on the same river reach, while in the parallel arrangement each reservoir is located on a separate river branch of the same order. In the cascade arrangement, the filling of a downstream reservoir depends on the amount of water held up by reservoirs further upstream and thus the cumulative effect on the storage capacities and drainage areas of all upstream reservoirs. On the other side, in the parallel arrangement, the filling and spilling processes are independent in each small reservoir. Figure S1 represents both arrangements.

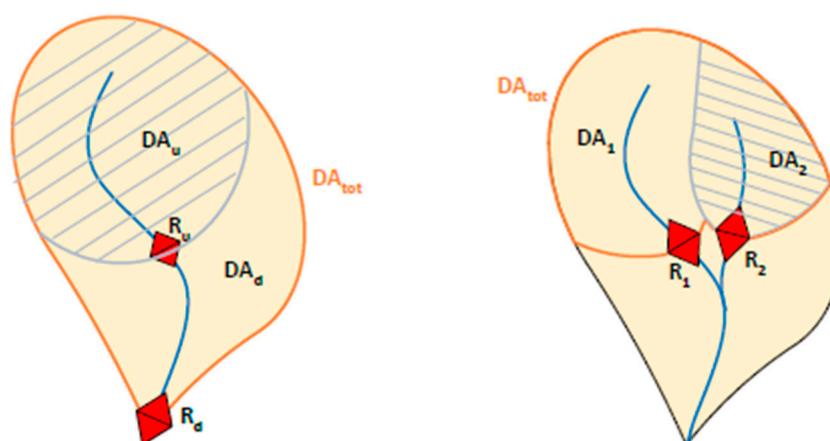


Figure S1. Schematic illustration of a sub-basin containing two small reservoirs configured in a cascade (left) and a parallel (right) arrangement. DA_{tot} : drainage area of the aggregated pond defining the total drainage fraction of the sub-basin; R_d/R_1 (red squares): downstream/first reservoir; R_u/R_2 (red squares): upstream/second reservoir; DA_d/DA_1 (not hatched): drainage area of downstream/first reservoir; DA_u/DA_2 (hatched in grey): drainage area of upstream/second reservoir; Blue line: river reaches.

A limitation to represent small reservoirs in SWAT is that the model allows only one single “pond” to be allocated to each sub-basin. It is common that many sub-basins ended up containing multiple small reservoirs after the watershed delineation. So, it is necessary to aggregate the small reservoirs into one “pond” in the model to represent the reservoir network. Based on the arrangement of small reservoirs and their drainage areas, Udinart et al. [1] developed certain calculation rules that were applied for the determination of the aggregated reservoir volume. The assumptions used in the aggregation methodology of the small reservoirs were:

1. Drainage areas of downstream reservoirs were kept fixed, while the volumes were reduced if necessary.
2. Only a fraction of the sub-basin contributes to runoff production that does not drain into any reservoir.
3. In the case that a pond is located directly at the outlet, no outflow from the sub-basin will occur until the storage capacity of the aggregated pond is exceeded.

Two extreme states may be distinguished regarding the storage effect:

1. The first state occurs when the entire amount of generated runoff in the system is stored. This threshold is the effective capacity of the small reservoir network. Only if a certain threshold water volume is exceeded the system spills. The effective capacity of the reservoir network determines whether it spills.
2. The second state occurs after some consecutive rainy days and after the full saturation of the system. At this point, the system only damps the outflow hydrograph, releasing the amount of water above the total storage capacity of the system. The total storage capacity determines how much water is spilled.
3. To simulate a storage effect that will match the one in reality on average, it was set the equivalent storage capacity of the lumped pond to a value in between effective capacity and total storage capacity.

Considering the cascade arrangement, if the relation of capacity to drainage area of an upstream reservoir is equal to or smaller than that of the downstream reservoir the upper dam will spill first. Hence, the equivalent storage capacity of the system amounts to the total capacity, the sum of both. In the case that this ratio is higher for the upstream reservoir, the downstream reservoir will spill first. In this case, the equivalent capacity is calculated as the sum between the full capacity of the reservoir with the larger specific drainage area and the other capacity reduced by the fraction of the two drainage areas. The following equations represent these processes.

$$\text{if } \frac{V(R_u)}{DA_u} \leq \frac{V(R_d)}{DA_d} : \quad (S1)$$

$$V_{eq} = V(R_u) + V(R_d) \quad (S2)$$

$$\text{if } \frac{V(R_u)}{DA_u} > \frac{V(R_d)}{DA_d} :$$

$$\text{if } DA_u > DA_d: V_{eq} = V(R_u) + \frac{DA_d}{DA_u} \cdot V(R_d) \quad (S3)$$

$$\text{if } DA_u < DA_d: V_{eq} = V(R_d) + \frac{DA_u}{DA_d} \cdot V(R_u) \quad (S4)$$

- V_{eq} : equivalent storage capacity of aggregated pond
- $V(R_u)$: storage capacity of upstream reservoir
- $V(R_d)$: storage capacity of downstream reservoir
- DA_u : drainage area of upstream reservoir
- DA_d : drainage area of downstream reservoir

Considering the parallel arrangement of small reservoirs in the same sub-basin, if the relation of capacity to drainage area of two reservoirs is equal both will spill at the same time. Hence, the equivalent storage capacity of the system amounts to the total capacity, the sum of both. For the case that this relation is smaller for one of the reservoirs, this dam will spill before the other one. In this case, the equivalent capacity is calculated as the sum of the full capacity of the reservoir with the larger specific drainage area and the other capacity reduced by the fraction of the two drainage areas. The following equations represent these processes.

$$\text{if } \frac{V(R_1)}{DA_1} \approx \frac{V(R_2)}{DA_2} : \quad (S5)$$

$$V_{eq} = V(R_1) + V(R_2) \quad (S6)$$

$$\text{if } \frac{V(R_1)}{DA_1} \neq \frac{V(R_2)}{DA_2} : \quad (S7)$$

$$\text{if } DA_1 > DA_2: V_{eq} = V(R_1) + \frac{DA_2}{DA_1} \cdot V(R_2) \quad (S8)$$

$$\text{if } DA_1 < DA_2: V_{eq} = V(R_2) + \frac{DA_1}{DA_2} \cdot V(R_1) \quad (S9)$$

- V_{eq} : equivalent storage capacity of aggregated pond
- $V(R_1)$: storage capacity of first reservoir
- $V(R_2)$: storage capacity of second reservoir
- DA_1 : drainage area of first reservoir
- DA_2 : drainage area of second reservoir

In case that multiple small reservoirs are arranged in the same configuration or that the two arrangements are combined in one sub-basin, it was started with the most upstream reservoirs. Their volumes were aggregated according to the respective rule, then this intermediate equivalent volume was again lumped with the small reservoir further downstream and so on. A detailed description of this methodology can be found in Rabelo et al. [1].

Table S1. Parameterization of reservoirs (water impoundments implemented into the model as reservoirs). Reservoir numbers and sub-basin numbers correspond to the IDs given automatically in ArcGIS. Abbreviations B., dC., and PP. stand for Benguê, Do Coronel, Poço da Pedra, respectively.

SWAT PARAMETER	MORES	IYRES	RES_ESA	RES_EVO	RES_PSA	RES_PVO	RES_VO	RES_K	EVRSV	IRESCO	RES_TAR	NDTARG		
			[10000 m ²]	[10000 m ³]	[10000 m ²]	[10000 m ³]	[10000 m ³]	[10000 m ³]			[mm/h]	[10000 m ³]	[d]	
EXPLANATION	RES-NO.	SB-NO.	Month, in which reservoir became operational	Year, in which reservoir became operational	Surface area when reservoir filled to emergency spillway	Storage volume when reservoir filled to emergency spillway	Surface area when reservoir filled to principle spillway	Storage volume when reservoir filled to principle spillway	Initial reservoir storage volume	Hydraulic conductivity of reservoir bottom	Lake evaporation coefficient	Reservoir outflow simulation code	Manually set target volume (equal for each months)	No. of days to reach target storage from current reservoir storage
2 (B.)	7	8	2000	438.00	2937.00	348.00	1956.00	85.80	0.0	1	2	1956.00	1.04	
1 (DC.)	70	1	1979	100.00	300.00	50.00	177.00	46.80	0.0	1	2	177.00	1.08	
0 (PP.)	108	11	1979	1639.00	14696.00	832.00	5200.00	1698.00	0.0	1	2	5200.00	2.25	
13	31	11	1994	22.56	66.91	15.89	38.19	0.00	0.1	1	2	38.19	1.01	
17	57	11	2003	40.10	107.62	26.89	57.67	0.00	0.1	1	2	57.67	1.00	
19	91	11	1999	23.13	63.44	15.77	34.66	0.00	0.1	1	2	34.66	1.00	
24	54	1	1991	7.47	13.66	4.08	5.07	0.00	0.1	1	2	5.07	1.00	
30	18	1	1979	21.93	62.36	15.17	34.66	3.47	0.1	1	2	34.66	1.00	
32	39	1	1979	12.39	32.56	8.31	17.10	1.71	0.1	1	2	17.10	1.00	
34	96	1	1979	10.96	24.59	6.82	11.34	1.13	0.1	1	2	11.34	1.00	
46	118	1	1979	89.26	307.50	64.40	192.83	38.57	0.1	1	2	192.83	1.14	
90	78	1	1979	14.75	37.03	9.71	18.78	1.88	0.1	1	2	18.78	1.00	
123	165	1	1979	20.74	64.12	14.82	37.55	3.75	0.1	1	2	37.55	1.00	
128	172	1	1979	4.19	12.91	2.97	7.56	0.00	0.1	1	2	7.56	1.00	
146	89	1	1979	45.03	127.57	30.74	71.06	7.11	0.1	1	2	71.06	1.05	
197	126	1	1979	8.91	18.15	5.25	7.60	0.00	0.1	1	2	7.60	1.00	
203	170	1	1979	5.97	8.26	2.39	2.15	0.00	0.1	1	2	2.15	1.00	

Table S2. Parameterization of ponds (water impoundments implemented into the model as ponds). Sub-basin numbers correspond to the IDs given automatically in ArcGIS.

SB NO.	DRAINAGE FRACTION	VOLUME PRINCIPLE (VPR)	SURFACE AREA PRINCIPLE (SAPR)	VOLUME EMERGENCY (VEM)	SURFACE AREA EMERGENCY (SAEM)	INITIAL STORAGE	NDTARG
	[-]	[10000 M ³]	[10000 M ²]	[10000 M ³]	[10000 M ²]	[10000 M ³]	[D]
2	0.1407	7.5238	8.6954	19.8279	16.2424	0	1
3	1	180.773	57.3379	245.2304	71.9404	36	1.35
5	0.4696	0.8167	1.8449	4.2163	5.1859	0	1
6	1	7.5132	9.6866	24.3122	23.3748	0	1
8	0.0391	3.7472	4.8147	10.7188	9.3316	0	1.23
10	0.4067	4.9495	7.3037	13.0763	15.9801	0	1
13	1	5.1694	5.8958	13.3848	10.7323	0	1
19	0.1458	0.8939	2.2837	5.4764	7.2734	0	1
23	0.0763	0.9053	1.9686	4.4619	5.374	0	1
25	1	1.2363	3.0887	7.26	9.4324	0	1
27	1	1.3203	2.4965	5.5348	6.1549	0	1
28	0.6053	1.5654	3.311	7.7935	9.624	0	1
33	1	0.3281	1.0389	2.6666	3.8864	0	1
34	0.3461	1.0677	2.184	4.8947	5.6966	0	1
36	0.2796	2.7783	3.9881	8.7843	8.2325	0	1
40	0.2456	0.2206	0.8092	2.2375	3.4799	0	1
43	1	2.3644	3.6029	7.9147	7.7095	0	1
46	0.8	0.6155	1.2528	3.2769	4.3561	0	1
59	0.8349	1.3593	3.6802	9.2813	12.9686	0	1
60	0.1041	0.1517	0.6393	1.9212	3.1614	0	1

SB NO.	DRAINAGE FRACTION	VOLUME PRINCIPLE (VPR)	SURFACE AREA PRINCIPLE (SAPR)	VOLUME EMERGENCY (VEM)	SURFACE AREA EMERGENCY (SAEM)	INITIAL STORAGE	NDTARG
	[-]	[10000 M ³]	[10000 M ²]	[10000 M ³]	[10000 M ²]	[10000 M ³]	[D]
61	1	5.4467	7.7014	17.1033	16.0336	0	1
64	0.3512	13.3103	10.6946	26.894	16.6532	1.3	1
65	1	5.1583	6.819	15.6876	14.6899	0	1
69	1	1.5008	4.0274	9.7324	13.1594	0	1
71	0.3748	1.1897	2.5641	6.0043	7.4319	0	1
72	0.819	1.7086	3.3212	7.5068	8.6277	0	1
74	0.4773	2.6592	4.4424	10.284	9.7641	0	1.22
75	0.0524	0.2017	0.8205	2.4195	3.9328	0	1
77	0.3162	2.9359	5.4036	12.6029	14.5969	0	1
79	0.2002	0.8926	2.127	4.9608	6.3172	0	1.02
80	0.4298	0.2251	1.0437	3.4006	5.8433	0	1
81	0.8215	0.6982	1.7436	4.1308	5.3995	0	1
82	0.7291	0.4968	1.5373	4.1508	6.2157	0	1
85	1	3.6826	4.7623	10.5934	9.2627	0	1
87	0.4025	2.9633	5.2069	11.8974	13.1948	0	1.05
88	0.95	0.7407	1.8059	4.2567	5.5043	0	1
90	1	1.5455	3.1587	7.5267	9.2915	0	1
93	1	7.5586	8.8378	20.4257	17.3195	0	1.02
95	0.7517	0.6823	1.6475	3.829	4.8805	0	1
98	0.4006	1.031	2.8662	7.3597	10.478	0	1
99	0.1782	0.7418	2.1364	5.2665	7.3392	0	1
100	1	29.5171	26.1075	64.7025	45.078	2.95	1.02
102	1	5.2178	7.5602	16.7197	15.8657	0	1
106	1	4.6885	5.7291	12.9448	11.0297	0	1
107	1	16.1602	13.5462	34.7797	24.2638	0	1
110	0.6358	6.8338	7.2609	17.1445	13.7023	0	1.04
112	0.0922	0.3418	1.066	2.7175	3.9329	0	1
113	0.3669	2.3219	5.0349	11.6349	14.25	0	1
117	0.081	4.1549	5.797	12.799	11.7793	0	1
119	0.1587	2.3743	4.6125	10.3382	11.7731	0	1
120	1	6.3861	8.8767	19.733	18.2893	0	1.05
122	1	3.6661	6.3311	14.2447	15.3917	0	1
123	1	4.1293	6.2352	14.076	14.1237	0	1
124	0.9464	2.5164	4.4371	10.3236	11.7177	0	1
128	0.1765	0.3815	1.1425	2.8614	4.0627	0	1
131	0.1025	0.578	1.4841	3.5123	4.6224	0	1
132	0.191	2.0321	4.3421	9.9109	11.9397	0	1
134	1	9.3236	13.8683	31.2845	31.0957	0	1
135	1	31.3789	22.0259	59.2537	34.1384	3.14	1.02
137	0.2512	1.0437	2.5996	6.1387	7.9998	0	1
138	0.1997	0.4126	1.2002	2.9704	4.1595	0	1
139	1	3.279	4.4266	9.7989	8.819	0	1
142	1	3.8062	5.6076	12.4575	12.0347	0	1
144	0.9	0.7818	1.7949	4.1178	5.1092	0	1
149	0.2237	0.1046	0.5058	1.6715	2.8961	0	1
150	0.5671	0.3025	0.9872	2.5698	3.7969	0	1
153	0.1916	25.1365	15.9595	44.4581	22.853	2.51	1.15
154	0.0536	0.0951	0.4765	1.6162	2.8355	0	1
157	0.1087	0.6223	1.8313	4.6474	6.6331	0	1.05
159	1	0.304	0.9902	2.5753	3.802	0	1
168	0.3333	0.7133	1.6942	3.9201	4.9533	0	1.19
171	0.9508	3.1478	4.3143	9.5366	8.6696	0	1
173	0.1667	0.2131	0.7918	2.2051	3.4481	0	1
175	1	2.5427	3.7716	8.2931	7.9395	0	1
181	1	2.6293	3.852	8.4747	8.0486	0	1
183	0.0633	5.2966	5.9868	13.6159	10.8486	0	1.07
187	0.0855	0.3322	1.0472	2.6822	3.9006	0	1
193	1	2.8781	4.0776	8.9893	8.3529	0	1
195	1	1.0286	2.1333	4.7922	5.6212	0	1

Table S3. Parameterization of calibrated model: Parameters set for the entire catchment.

ENTIRE CATCHMENT	
CALIBRATED PARAMETER	Calibrated Value
GW_DELAY	12 d and 30 d
CH_K1	5 mm/h to 72 mm/h
TRNSRCH	0.3
OV_N	0.6
CN2	57.34 to 92
CH_N1	0.065
CANMX	1.5

Table S4. Parameterization of calibrated model: Parameters set for specific sub-basins of the catchment. Distinction between sub-catchments of two strategic reservoirs and topographic position of sub-basins.

CALIBRATED PARAMETER	SUB-CATCHMENTS			SPECIFIC SUB-BASINS			
	Poço da Pedra Catchment	Benguê Catchment	Upstream SB	Transition SB/Medium-order Reaches	Downstream SB/High-order Reaches	Lowlands (incl. Do Coronel Sub-catchment)	
	Calibrated Values						
REVAPMN	265	265	265	265	265	265	
GW_REVAP	0.15	0.15	0.25	0.15	0.1	0.25	
GWQMN	700	700	700	700	700	700	
CH_K2	25	19	5	20	72	72	
CH_N2	0.05	0.05	0.05	0.05	0.05	0.05	
SURLAG	4	4	4	4	4	4	
ALPHA_BF	0.8	0.8	0.8	0.8	0.8	0.8	
RCHARG_DP	0.25	0.25	0.25	0.25	0.25	0.25	
ALPHA_BNK	0.6	0.6	0.6	0.6	0.6	0.6	

Table S5. Parameterization of calibrated model: Parameters set for specific zones in the catchment. Distinction between soil types.

CALIBRATED PARAMETERS	SOIL TYPE				
	Bruno	Latosol	LitolicosEu	Planosolos	Podisolico-EqEu
	Calibrated Values				
ESCO	0.02	0.02	0.02	0.02	0.02
LAT_TTIME	0	0	0	0	0
SOL_K	PTF results*	PTF results*	PTF results* x 0.8	PTF results*	PTF results*
SOL_AWC	PTF results*	PTF results*	PTF results* x 1.2	PTF results*	PTF results*
GW_REVAP	0.1 and 0.15	0.15	0.1, 0.15 and 0.25	0.25	0.1 and 0.15
SOL_CRK	0.3	0.4	0.3	0.01	0.01

*PTF results can be found at Rabelo et al. [1].

Table S6. - Annual streamflow in m³/s at Malhada Station for each increase in the number of small reservoirs per year of simulation.

Year	Observed by COGERH	Reference model	Annual Streamflow [m ³ /s]							
			Simulation models - Small reservoirs per area of catchment							
			0.1/km ²	0.25/km ²	0.5/km ²	0.75/km ²	1.0/km ²	1.5/km ²	2.0/km ²	3.0/km ²
1981	699.5	1828.8	1820.8	1786.5	1739.5	1712.3	1689.0	1661.9	1638.2	1604.6
1982	204.9	275.9	274.1	268.5	261.0	256.7	253.0	248.0	244.0	237.8
1983	11.9	41.6	40.9	39.0	37.3	36.4	35.7	34.5	33.6	32.6
1984	951.3	507.1	504.6	496.3	486.1	479.2	472.7	463.7	456.0	445.9
1985	5905.2	4303.6	4295.6	4257.2	4200.7	4155.6	4113.5	4052.6	3990.6	3897.3
1986	1791.5	1771.4	1765.4	1753.3	1741.2	1730.4	1722.8	1707.4	1694.4	1673.2
1987	699.8	571.4	570.6	564.4	558.2	552.5	548.5	540.2	533.8	524.6
1988	839.4	680.8	677.8	666.8	655.3	648.1	640.9	623.5	612.8	598.1
1989	3373.2	1830.7	1827.1	1811.3	1793.7	1783.0	1772.3	1756.2	1736.2	1708.3
1990	236.2	209.9	209.3	207.6	205.2	203.8	202.3	199.5	197.7	194.9
1991	52.5	8.1	7.9	7.7	7.6	7.5	7.4	7.3	7.2	7.1
1992	347.7	560.3	557.3	547.8	536.0	527.4	521.4	512.8	505.8	493.3
1993	1.3	11.7	11.3	10.7	10.5	10.3	10.2	10.0	9.9	9.8
1994	73.3	154.4	152.8	149.3	145.7	142.2	139.2	136.8	133.0	128.8
1995	816.0	478.8	476.1	467.8	458.6	451.4	445.1	436.6	429.5	419.4
1996	97.1	175.9	174.9	170.8	166.5	163.2	160.2	156.6	153.9	148.5
1997	1219.2	486.0	483.5	473.1	461.5	453.5	448.1	439.8	431.6	418.0
1998	21.7	16.7	16.4	15.3	14.3	13.7	13.3	12.7	12.3	11.7
1999	115.1	154.3	153.2	148.2	140.6	134.3	130.0	124.8	122.1	118.1
2000	240.4	316.8	314.4	307.5	299.9	294.0	289.9	282.3	275.5	265.4
2001	0.0	7.0	7.0	6.9	6.9	6.8	6.8	6.7	6.7	6.6
2002	288.4	369.7	367.7	360.2	351.2	346.4	341.0	333.6	327.7	321.1
2003	109.1	131.5	130.2	125.8	120.5	117.2	115.5	112.2	110.2	107.1
2004	3741.3	4157.2	4147.6	4112.1	4066.7	4033.0	4006.5	3959.5	3915.8	3841.6
2005	44.7	33.9	33.0	31.2	28.9	27.5	26.2	25.1	24.4	23.8
2006	195.0	476.8	474.0	463.9	454.0	444.5	439.1	428.0	418.3	404.7
2007	454.8	814.5	810.3	799.2	784.1	774.8	767.7	755.4	743.4	726.7
2008	332.9	743.9	740.1	729.7	719.4	711.1	704.8	694.5	685.4	673.4
2009	895.9	1334.6	1324.2	1302.7	1276.3	1257.6	1244.9	1220.2	1202.2	1173.6
2010	38.1	130.4	129.4	123.7	118.6	116.5	115.5	113.2	110.9	107.6

Table S7. - Percentage of annual streamflow reduction at Malhada Station for each increase in the number of small reservoirs per year of simulation.

Year	Reference model	Annual Percentage of Flow Reduction							
		Simulation models - Small reservoirs per area of catchment							
		0.1/km ²	0.25/km ²	0.5/km ²	0.75/km ²	1.0/km ²	1.5/km ²	2.0/km ²	3.0/km ²
1981	-	-0.4%	-2.3%	-4.9%	-6.4%	-7.6%	-9.1%	-10.4%	-12.3%
1982	-	-0.6%	-2.7%	-5.4%	-6.9%	-8.3%	-10.1%	-11.6%	-13.8%
1983	-	-1.6%	-6.2%	-10.4%	-12.6%	-14.2%	-17.1%	-19.2%	-21.5%
1984	-	-0.5%	-2.1%	-4.1%	-5.5%	-6.8%	-8.6%	-10.1%	-12.1%
1985	-	-0.2%	-1.1%	-2.4%	-3.4%	-4.4%	-5.8%	-7.3%	-9.4%
1986	-	-0.3%	-1.0%	-1.7%	-2.3%	-2.7%	-3.6%	-4.3%	-5.5%
1987	-	-0.1%	-1.2%	-2.3%	-3.3%	-4.0%	-5.5%	-6.6%	-8.2%
1988	-	-0.4%	-2.1%	-3.7%	-4.8%	-5.9%	-8.4%	-10.0%	-12.2%
1989	-	-0.2%	-1.1%	-2.0%	-2.6%	-3.2%	-4.1%	-5.2%	-6.7%
1990	-	-0.3%	-1.1%	-2.2%	-2.9%	-3.6%	-5.0%	-5.8%	-7.1%
1991	-	-2.4%	-4.1%	-6.0%	-7.5%	-8.5%	-9.8%	-10.9%	-12.1%
1992	-	-0.5%	-2.2%	-4.3%	-5.9%	-6.9%	-8.5%	-9.7%	-12.0%
1993	-	-3.5%	-8.7%	-10.5%	-11.7%	-12.6%	-14.0%	-14.9%	-15.9%
1994	-	-1.0%	-3.3%	-5.6%	-7.9%	-9.8%	-11.4%	-13.8%	-16.6%
1995	-	-0.6%	-2.3%	-4.2%	-5.7%	-7.0%	-8.8%	-10.3%	-12.4%
1996	-	-0.6%	-2.9%	-5.4%	-7.2%	-9.0%	-11.0%	-12.5%	-15.6%
1997	-	-0.5%	-2.6%	-5.0%	-6.7%	-7.8%	-9.5%	-11.2%	-14.0%
1998	-	-2.0%	-8.6%	-14.5%	-18.0%	-20.6%	-24.0%	-26.4%	-30.3%
1999	-	-0.8%	-4.0%	-8.9%	-13.0%	-15.8%	-19.2%	-20.9%	-23.5%

2000	-	-0.8%	-2.9%	-5.4%	-7.2%	-8.5%	-10.9%	-13.0%	-16.2%
2001	-	-0.6%	-1.6%	-2.5%	-3.2%	-3.7%	-4.5%	-5.1%	-5.8%
2002	-	-0.5%	-2.6%	-5.0%	-6.3%	-7.8%	-9.8%	-11.4%	-13.2%
2003	-	-1.0%	-4.4%	-8.4%	-10.9%	-12.2%	-14.7%	-16.2%	-18.6%
2004	-	-0.2%	-1.1%	-2.2%	-3.0%	-3.6%	-4.8%	-5.8%	-7.6%
2005	-	-2.5%	-8.1%	-14.6%	-18.9%	-22.6%	-26.0%	-27.9%	-29.8%
2006	-	-0.6%	-2.7%	-4.8%	-6.8%	-7.9%	-10.3%	-12.3%	-15.1%
2007	-	-0.5%	-1.9%	-3.7%	-4.9%	-5.7%	-7.3%	-8.7%	-10.8%
2008	-	-0.5%	-1.9%	-3.3%	-4.4%	-5.3%	-6.6%	-7.9%	-9.5%
2009	-	-0.8%	-2.4%	-4.4%	-5.8%	-6.7%	-8.6%	-9.9%	-12.1%
2010	-	-0.8%	-5.2%	-9.1%	-10.6%	-11.5%	-13.2%	-14.9%	-17.5%
