

## Article

# Comparison of Critical Shear Stress of Rill Erosion Estimated from Two Methods

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**Abstract:** Various disputes exist regarding the critical shear stress ( $\tau_c$ ) of rill erosion determined by linear regression. Alternatively, some researchers have attempted to measure critical shear stress ( $\tau_o$ ) of rill erosion by observing the start of soil particle detachment and showed promising results. However, few studies have been conducted to evaluate the reliability of the method determining  $\tau_o$ . Thus, this study was conducted to identify the reliability of the method determining  $\tau_o$  by comparing the values of  $\tau_c$  and  $\tau_o$  and their relationships with rill erodibility ( $K_r$ ) and influencing factors, using 360 disturbed soil samples (standing for the freshly tilled condition) from six sampling sites along a 330 km transect in the Loess Plateau. Results indicated that the mean  $\tau_c$  was 1.90 times greater than that of  $\tau_o$ . No significant relationship was found between  $\tau_c$  and  $K_r$ . An inverse changing trend between  $K_r$  and  $\tau_o$  was observed from Yijun to Zizhou, while both  $K_r$  and  $\tau_o$  exhibited increasing trends from Zizhou to Yulin. The  $\tau_o$  of six sampling sites all demonstrated increasing trends with slope gradient ( $S$ ), which contradicted the statement of the WEPP model that  $\tau_c$  was independent of  $S$ . The relationships between  $\tau_c$  and the measured soil properties were poor, whereas the  $\tau_o$  increased exponentially with soil organic matter. Generally,  $\tau_o$  did not present significantly better results than  $\tau_c$ ; the reliability of  $\tau_o$  also showed some uncertainties, such as the subjective judgment of the beginning detachment of soil particles. This result has great importance for deepening our understanding of the rill erosion mechanism.

**Keywords:** soil erosion; critical shear stress; rill erodibility; soil detachment capacity; Loess Plateau



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## 1. Introduction

Soil erosion by water leads to land degradation, reduced crop yields, water pollution, threatening biodiversity and sustainability of environmentally protected areas, and other problems, which have made it a serious global problem [1–5]. A soil erosion model is essential for understanding the erosion process and implementing effective soil conservation measures [6–8]. The Universal Soil Loess Equation (ULSE) and revised ULSE (RULSE) were widely applied to predict soil erosion [5]. However, ULSE and RULSE were empirical and lacked deep recognition of erosion processes and erosion mechanisms [5,8]. Thus, many studies in recent decades have been concerned with process-based models, which were physically-based and could model erosion processes [9,10]. In these models, represented by the Water Erosion Prediction Project (WEPP), the rill erosion of overland flow is the most important process that influences sediment production [11]. Rill erodibility ( $K_r$ ,  $s\ m^{-1}$ ) and critical shear stress ( $\tau_c$ , Pa) (both refer to soil resistance to overland flow) are crucial parameters for modeling rill erosion [7].  $K_r$  and  $\tau_c$  are usually determined by the linear regression of soil detachment capacity ( $D_c$ ,  $kg\ m^{-2}\ s^{-1}$ ) and flow shear stress ( $\tau$ , Pa) [12].

Generally,  $K_r$  and  $\tau_c$  are greatly influenced by soil properties (such as soil texture, soil cohesion, soil organic matter (SOM), plant roots, etc.) [13–15].

In past decades, researchers have done significant work on the influencing factors of  $K_r$  and have reached a consensus about the effects of these influencing factors on  $K_r$  [16–18]). Soil texture was widely used in the prediction of  $K_r$  [16]. Geng et al. [18] studied the response of  $K_r$  to landscape positions in a small watershed and found  $K_r$  decreased and increased with increasing clay content and sand content, respectively. Soil cohesion reflects the cohesive forces among soil particles [19]. Wang et al. [20] found a negative relationship between  $K_r$  and soil cohesion. SOM affects soil resistance to overland flow by functioning as a cementing agent among soil particles [17]. Geng et al. [21] proposed a significant correlation between  $K_r$  and SOM. Plant roots also impose great influence on  $K_r$  due to their chemical bonding effect and physical bonding effect [14,22].  $K_r$  usually increases exponentially with root mass density [20].

Significant disagreement remains, however, regarding other crucially important parameters of soil resistance to overland, namely  $\tau_c$  [23,24]. Generally speaking, these disagreements can be categorized as follows: (1) Unlike  $K_r$ , the relationships between  $\tau_c$  and its potential influencing factors were rather confusing [21,25]. For example, Gilly et al. [25] quantified the  $\tau_c$  of 36 soils throughout the United States and indicated that  $\tau_c$  could be predicted by clay content, linear extensibility coefficient, and soil water content. Nonetheless, Geng et al. [21] showed no significant relationship between  $\tau_c$  and 17 measured soil properties for 36 soil types across Eastern China. (2) There is also no general agreement on the relationship between  $K_r$  and  $\tau_c$ . Many studies indicated that soils with higher  $\tau_c$  correspond to less erodibility [26,27]. Knapen et al. [17] found that no significant relationship existed between  $K_r$  and  $\tau_c$  for both laboratory ( $n = 179$ ) and field experiments ( $n = 151$ ). Additionally, Mamo and Bubbenzer's [27] findings did not support the opinion that when  $\tau_c$  was smaller,  $K_r$  would be larger. (3) In the WEPP,  $\tau_c$  does not change with the variation of slope gradient ( $S$ ) and hence is considered to be a constant [28]. Nevertheless, in developing a rational method for predicting  $\tau_c$ , Lei et al. [29] suggested that  $\tau_c$  increased when  $S$  changed from 0.087 to 0.423 and attributed this increase to the fact that the force along the slope enhanced when  $S$  was raised. Moreover, the critical shear stress of sediment particles in rivers mostly depended on bed  $S$  [30,31].

Due to the limitations of determining critical shear stress in rills by linear regression [32,33], some researchers have tried to determine critical shear stress by gradually increasing flow shear stress until an obvious detachment of soil particles is observed [34,35]. The critical shear stress of rill erosion determined by this method was called  $\tau_o$ . Nouwakpo et al. [36] considered  $\tau_c$  was not capable of detecting small variations in real critical shear stress because of its projected characteristic. Thus, they determined the  $\tau_o$  of different hydraulic gradients and demonstrated a good linear relationship with the hydraulic gradient that was in line with the changing trend of  $K_r$ . Lei and Nearing [34] designed an apparatus to control flow discharge for well determining the  $\tau_o$  of loose material. Although these studies presented satisfactory results, they only used the method of observing the initial detachment of soil particles to determine  $\tau_o$  and did not compare these results with the widely used  $\tau_c$  [17,21]. Thus, it is of paramount importance to investigate whether the method determining  $\tau_o$  in rills is reliable.

Different from determining  $\tau_o$  in rills, the method of determining critical shear stress in rivers by observing initial sediments has been extensively used [37–39], and a great deal of knowledge has been gained [40–42]. However, there are distinct discrepancies in hydraulic environments between rivers and rills [43]. Typical flow depths in rills are several centimeters or less, which is several orders of magnitudes lower than that of rivers [7]. The  $S$  of rivers was generally gentle (usually less than 0.01) [31], while it was steep for rills (up to 0.46) [44]. What is more, distinguishing differences exist between soils in rills and sediments in rivers [20,31]. Considering the great discrepancies between rivers and rills, the knowledge obtained from rivers may not be valid in rills [7,43]. Therefore, the reliability of the method determining  $\tau_o$  in rills is in need of exploration.

For the purpose of exploring the reliability of the method determining  $\tau_o$  in rills, undisturbed soils were preferable, considering the extrapolation of research results [24,45]. However, as mentioned above,  $K_r$  and  $\tau_c$  of undisturbed soils were affected by numerous

soil properties and plant roots [17]. These influencing factors of  $K_r$  and  $\tau_c$  were various, complicated, and, at times, greatly correlated [21]. Specifically designed experiments, which controlled influencing factors, were usually aimed at establishing an accurate relationship between  $K_r$  and  $\tau_c$  and their influencing factors [21,27,46]. Clear relationships between  $\tau_o$  and  $\tau_c$  and their influencing factors were pivotal to estimating the reliability of the method determining  $\tau_o$ . Disturbed soils, compared with undisturbed soils, could well control influencing factors, whereby the clear relationships between  $\tau_o$  and  $\tau_c$  and some fundamental factors (e.g., soil texture and SOM) could be more easily established with relatively lesser soil samples [24,47]. Therefore, disturbed soils, standing for the freshly tilled condition, were used in this study [21].

As mentioned earlier, a lot of disagreement exists regarding the usually used method (linear regression) to determine the critical shear stress of rills ( $\tau_c$ ), whereas the method measuring critical shear stress of rills by observing the start of soil particle detachment ( $\tau_o$ ) has shown promising results [36]. However, the reliability of the method determining  $\tau_o$  was still unknown. Therefore, this study aimed to identify the reliability of the method determining  $\tau_o$  in rills by comparing (1) the values of  $\tau_o$  and  $\tau_c$ , (2) the relationship between  $\tau_o$  and  $\tau_c$ , and  $K_r$ , and (3) the relationship between  $\tau_o$  and  $\tau_c$  and their influencing factors. This study's findings have important implications for recognizing rill erosion mechanisms and enhancing the prediction accuracy of erosion models.

## 2. Material and Methods

### 2.1. Soil Sampling

The Loess Plateau (total area of  $6.2 \times 10^5 \text{ km}^2$ ) is located in northwestern China. Irrational land use and low coverage of vegetation have made it the most severely eroded region in the world, and the soil erosion rate could be greater than  $59,700 \text{ t km}^{-2} \text{ yr}^{-1}$  [48]. The Loess Plateau has a temperate and monsoon climate, with annual precipitation and mean temperature changes ranging from 800 mm to less than 200 mm and from  $14.3 \text{ }^\circ\text{C}$  to  $3.6 \text{ }^\circ\text{C}$ , respectively, from the south to the north. The vegetation zones change from forest to steppe desert from the south to the north [49]. Thick loess–paleosol deposits are distributed throughout the Loess Plateau. Clay loam is the most widespread soil, with soil texture becoming more sand and less clay from the south to the north [50]. Additionally, the SOM followed a decreasing trend from south to north [51]. From the south to the north, the decreasing clay content and SOM and the increasing sand content in this region would lead to an increase in  $K_r$  and possibly to the corresponding changes in  $\tau_o$  and  $\tau_c$  [21]. These influencing factors of  $K_r$  and  $\tau_c$  also showed great variations from south to north [21], which was helpful for establishing accurate relationships between critical shear stresses and their influencing factors. Thus, a line transect with 330 km was laid out from the south to the north in this region (Figure 1). This transect has spread across three vegetation zones: forest, forest steppe, and steppe. Six sampling sites on this transect were chosen, and the interval of the two adjacent sampling sites was about 66 km (Figure 1 and Table 1). The southernmost sampling site was Yijun (a typical forest zone), and the northernmost sampling site was Yulin, the north part of which was the Mu Us desert.

Disturbed soils of cropland (20 cm) on the six sampling sites were selected from April to May 2019. On each sampling site, 100 kg of disturbed soils (top 20 cm of soil layer) were collected in 'S' shape (six soil samples). These soil samples were air-dried and sieved with a mesh aperture of 2 mm to simulate the freshly tilled condition [18,52]. A portion of these disturbed soils was used for the measurement of critical shear stress, and the other portion was used for the determination of soil properties.

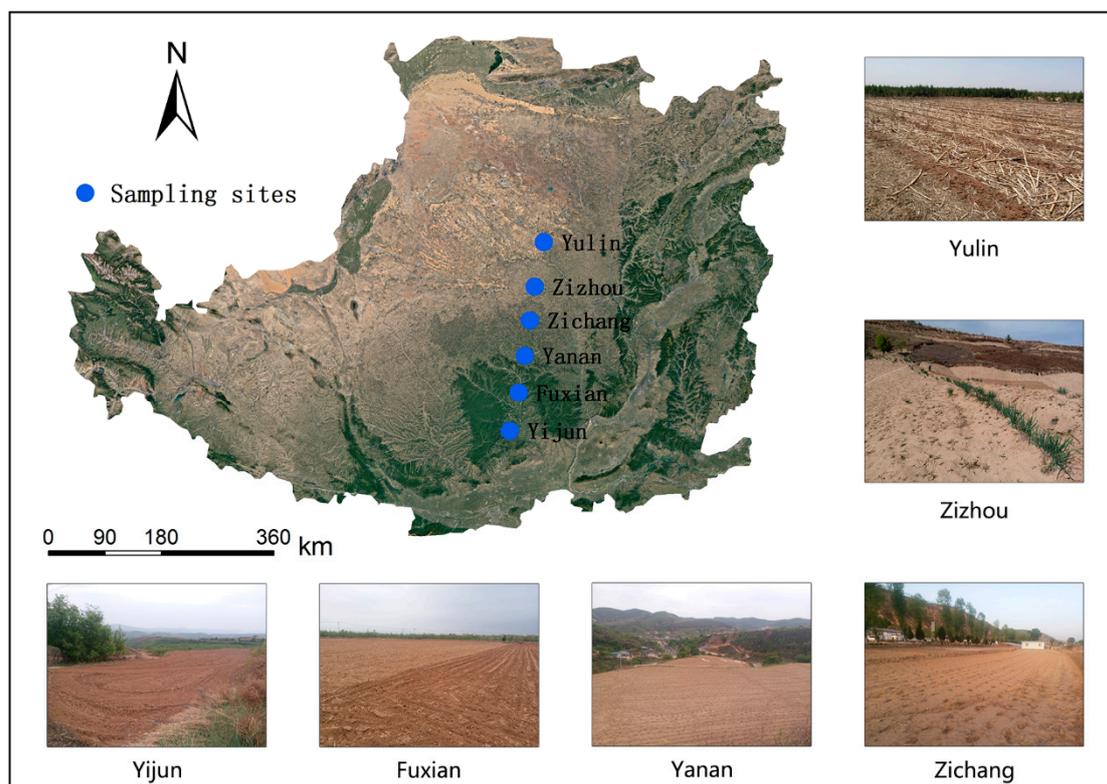


Figure 1. The six sampling sites.

Table 1. Description of six sampling sites.

Sampling Site	Longitude	Latitude	Elevation (m)	Temperature (°C)	Precipitation (mm)	Coverage (%)	Crop
Yijun	109°22'40"	35°23'22"	1002	10.3	591	1	Corn
Fuxian	109°27'19"	35°57'50"	1183	10.2	542	2	Corn
Yanan	109°34'34"	36°40'37"	980	9.9	514	5	Corn
Zichang	109°40'15"	37°10'31"	1058	9.6	437	3	Corn
Zizhou	109°44'19"	37°42'40"	1077	9.3	411	5	Corn
Yulin	109°48'10"	38°12'42"	1132	8.8	383	3	Corn

## 2.2. Determination of Critical Shear Stress

The water content ( $150 \text{ g kg}^{-1}$ ) of sieved soils was obtained by a sprinkling can, and the water was equilibrated for 24 h in a sealed plastic bucket. The same water content was used for the six sampling sites to reduce the effect of the water content and to fill the soil easily [21]. The wetted soils were put into a stainless-steel box (10 cm in diameter) in several steps. The bulk density of the samples from the six sampling sites was kept at  $1.19 \text{ g kg}^{-1}$  (i.e., the average bulk density of six sampling sites). For each sampling site, 60 soil samples were made, half of which was used for the determination of  $\tau_c$ , and the other half was tested for  $\tau_o$ . Altogether, 360 soil samples were collected for all six sampling sites. Before the measurement of critical shear stress, these soil samples were saturated for 12 h and then drained for 12 h.

$\tau_o$  was determined in a hydraulic flume (0.35 m in width, 5 m in length), which was described thoroughly in Su et al. [53]. To explore the effect of S on  $\tau_o$ , six S (3.49%, 8.72%, 17.36%, 25.88%, 34.20%, and 42.26%) were set. On each S,  $\tau_o$  was determined by gradually increasing the flow discharge until the continuous detachment of soil particles was observed [36,54]. A series of valves was employed to adjust the flow discharge ( $Q$ ,  $\text{m}^3 \text{ s}^{-1}$ ). The increase of  $Q$  was stopped when the continuous detachment of soil particles occurred. The corresponding  $Q$ , flow velocity ( $V$ ,  $\text{m s}^{-1}$ ), and flow depth ( $H$ , m)

at this moment were termed as critical flow discharge ( $Q_o$ ,  $\text{m}^3 \text{s}^{-1}$ ), critical flow velocity ( $V_o$ ,  $\text{m s}^{-1}$ ), and critical flow depth ( $H_o$ , m). The measurements of  $Q_o$ ,  $V_o$ , and  $H_o$  were the same as those obtained by Wang et al. [20] and Geng et al. [55], which have been thoroughly described.  $\tau_o$  of each soil sample was calculated based on the following Equation (1):

$$\tau_o = \rho g H_o S \quad (1)$$

where  $\rho$  ( $\text{kg m}^{-3}$ ) and  $g$  ( $\text{m s}^{-2}$ ) are the water density and the gravity constant, respectively. The mean  $\tau_o$  of five duplicates under the particular S was used. Table 2 lists the statistical characteristics of the noted hydraulic parameters.

**Table 2.** Critical hydraulic parameters for  $\tau_o$  test of six sampling sites.

Items	Minimum	Maximum	Mean	Standard Deviation	CV
$Q_o$ ( $\text{m}^3 \text{s}^{-1}$ )	0.000040	0.000140	0.000065	0.000039	0.59
$H_o$ (m)	0.00060	0.00133	0.00078	0.00028	0.35
$V_o$ ( $\text{m s}^{-1}$ )	0.18	0.25	0.21	0.03	0.13
$\tau_o$ (Pa)	1.08	1.78	1.34	0.24	0.18

Note:  $Q_o$ , critical flow discharge of  $\tau_o$  test;  $H_o$ , critical flow depth of  $\tau_o$  test;  $V_o$ , critical flow velocity of  $\tau_o$  test; CV, coefficient of variance.

Similar to  $\tau_o$ ,  $\tau_c$  was measured in the same hydraulic flume. The S was kept at a constant value (25.88%) to eliminate the influence of S. Six flow discharges ranging from  $2.5 \times 10^{-4}$  to  $2.5 \times 10^{-3} \text{ m}^3 \text{ s}^{-1}$  were applied to determine  $D_c$ . After the desired Q with an error of less than 2% was set, V and H were measured. The detailed measuring procedures of Q, V, and H were similar to  $Q_o$ ,  $V_o$ , and  $H_o$ .  $\tau$  of each flow discharge was calculated using Equation (1), but the difference was that  $\tau_o$  and  $H_o$  were replaced by  $\tau$  and H, respectively. The corresponding hydraulic parameters for each flow discharge are listed in Table 3. Then, the measurement of  $D_c$  was taken when the soil sample was put into the sample hole and was stopped when the scouring depth of 2 cm was exceeded. The scoured soil samples were oven-dried for 24 h ( $105^\circ \text{C}$ ).  $D_c$  was calculated by the following equation:

$$D_c = \frac{M_i - M_f}{AT} \quad (2)$$

where  $M_i$  is initial dry soil mass;  $M_f$  is final dry soil mass; A is soil sample area; and T is scouring time. Five duplicates of each flow discharge were measured, and the mean values of five duplicates were used.  $K_r$  and  $\tau_c$  were obtained by regressing six pairs of  $D_c$  and  $\tau$  by Equation (3). The regressing slope and intercept on the X-axis were  $K_r$  and  $\tau_c$ , respectively (Table 4).

$$D_c = K_r(\tau - \tau_c) \quad (3)$$

**Table 3.** Hydraulic parameters for  $\tau_c$  test.

Q ( $\text{m}^3 \text{s}^{-1}$ )	V ( $\text{m s}^{-1}$ )	H (m)	$\tau$ (Pa)
0.00025	0.53	0.0014	3.45
0.0005	0.73	0.0020	5.00
0.001	1.01	0.0028	7.20
0.0015	1.26	0.0034	8.62
0.002	1.42	0.0040	10.22
0.0025	1.61	0.0044	11.26

**Table 4.** Calculated results of soil erosion resistance to overland flow of six sampling sites.

Sampling Sites	Regression Equations	R <sup>2</sup>	NSE
Yijun	$D_c = 0.31 (\tau - 1.59)$	0.88	0.88
Fuxian	$D_c = 0.30 (\tau - 2.96)$	0.95	0.95
Yanan	$D_c = 0.33 (\tau - 2.44)$	0.98	0.98
Zichang	$D_c = 0.35 (\tau - 2.17)$	0.96	0.98
Zizhou	$D_c = 0.65 (\tau - 2.48)$	0.98	0.98
Yulin	$D_c = 1.97 (\tau - 3.65)$	0.98	0.97

So as to explore the influencing factors of  $\tau_o$  and  $\tau_c$ , some basic influencing factors (e.g., soil texture and SOM) were measured. The measurements of soil texture (e.g., clay content, silt content, and sand content) (two duplicates) and SOM (two duplicates) were taken by hydrometer and potassium dichromate colorimetric, respectively. The measured soil properties of six sampling sites are given in Table 5. As shown in Table 5, the clay content, silt content, and SOM generally followed decreasing trends from south to north, whereas sand content followed an increasing trend. Additionally, the soil properties of the six sampling sites presented great variations (Table 5).

**Table 5.** Soil properties of six sampling sites.

Sampling Site	Clay Content (%)	Silt Content (%)	Sand Content (%)	SOM (g kg <sup>-1</sup> )
Yijun	10.11	77.36	12.53	12.37
Fuxian	9.47	76.66	13.87	12.69
Yanan	13.55	76.67	9.79	6.05
Zichang	11.56	62.33	26.11	6.37
Zizhou	5.43	39.97	54.60	1.19
Yulin	1.12	4.68	94.20	0.59

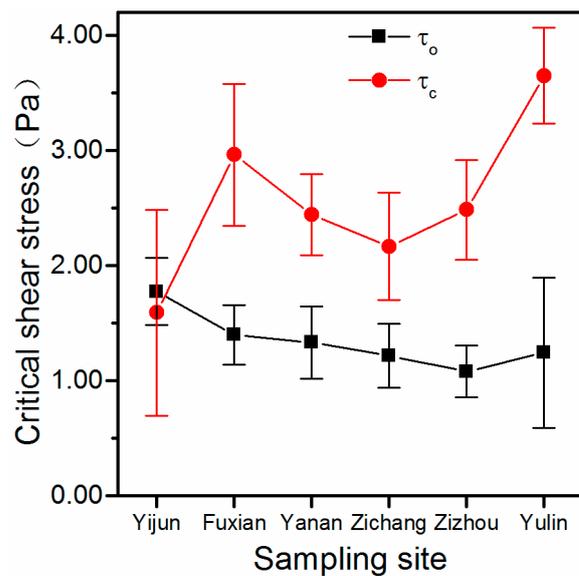
### 2.3. Data Analysis

All statistical analyses were conducted in SPSS 23.0. The difference between  $\tau_o$  and  $\tau_c$  was detected by the paired sample *t*-test. Pearson correlation analysis was applied to detect the relationship between measured soil properties and  $K_r$ ,  $\tau_o$ , and  $\tau_c$ . Linear regression or nonlinear regression was applied to calculate  $K_r$  and  $\tau_c$  and to analyze the relationships among  $S$ , soil properties, and  $\tau_o$ . The coefficient of determination ( $R^2$ ) and Nash–Sutcliffe efficiency (NSE) was employed to evaluate the fitness of the regressing results.

## 3. Results

### 3.1. Comparison of the Values of $\tau_o$ and $\tau_c$

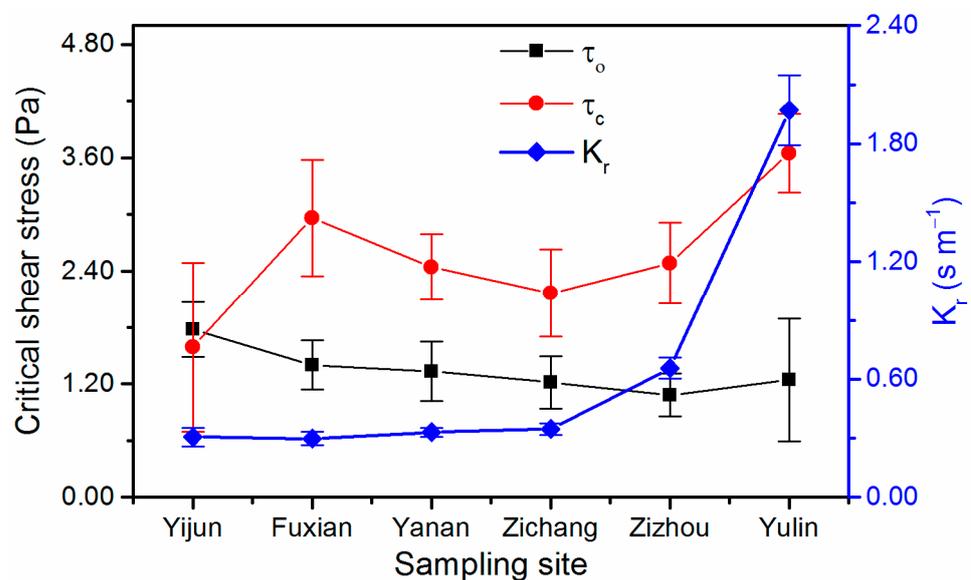
The minimum, mean, and maximum  $\tau_c$  of the six sampling sites were 1.59 Pa, 2.55 Pa, and 3.65 Pa, respectively, while  $\tau_o$  varied from 1.08 Pa to 1.78 Pa with a mean of 1.34 Pa. On average, the  $\tau_c$  was 1.90 times greater than that of  $\tau_o$ .  $\tau_c$  and  $\tau_o$  both showed moderate variability with a coefficient of variance (CV) of 0.25 and 0.16, respectively. As shown in Figure 2, the  $\tau_c$  of all sampling sites was greater than  $\tau_o$ , except for Yijun, where  $\tau_o$  was only slightly greater than  $\tau_c$ . The paired sample *t*-test also demonstrated a significant difference between  $\tau_o$  and  $\tau_c$  for six sampling sites ( $p < 0.05$ ). Figure 2 shows that the changing trend of  $\tau_c$  was chaotic from south to north. The two largest values appeared at Fuxian and Yulin, whereas the two lowest values appeared at Yijun and Zichang. From south to north, however,  $\tau_o$  decreased gradually from Yijun (the maximum  $\tau_o$ ) to Zizhou (the minimum  $\tau_o$ ) and increased from Zizhou to Yulin.



**Figure 2.** Comparison of critical shear stress determined by observed soil particles detachment ( $\tau_o$ ) and critical shear stress measured by linear regression ( $\tau_c$ ).

3.2. Comparison of the Relationship between  $\tau_c$  and  $\tau_o$ , and  $K_r$

Table 4 lists the calculated results of  $K_r$  and  $\tau_c$ , which showed satisfactory results with  $R^2$  (0.88–0.98) and NSE (0.88–0.98).  $K_r$  generally followed an increasing trend from south to north (Figure 3). The increasing trend of  $K_r$  was associated with variations in soil texture (Table 6).  $K_r$  decreased with clay content and silt content, and it increased with sand content.



**Figure 3.** Comparison of the relationships between rill erodibility ( $K_r$ ) and  $\tau_o$  and  $\tau_c$ .

**Table 6.** Relationship between  $K_r$  and soil properties by Pearson Correlation Coefficient.

Items	Clay Content (%)	Silt Content (%)	Sand Content (%)	SOM ( $g\ kg^{-1}$ )
$K_r$	−0.884 *	−0.946 **	0.947 **	−0.687

Note: \*\* and \* indicate significance at the 0.05 level and 0.01 level, respectively.

As shown in Figure 3, at the sampling site of Yulin, both  $K_r$  and  $\tau_c$  had the maximum value. Yijun, however, had the lowest  $K_r$  and nearly the lowest  $\tau_c$ . Fuxian had the lowest  $K_r$  and the second-largest  $\tau_c$ . No significant relationship existed between  $K_r$  and  $\tau_c$  since a very different changing trend was observed between  $\tau_c$  and  $K_r$  (Figure 3). For the relationship between  $K_r$  and  $\tau_o$ , from Yijun to Yulin,  $K_r$  and  $\tau_o$  followed increasing and decreasing trends, respectively. From Zizhou to Yulin, however, both  $K_r$  and  $\tau_o$  followed increasing trends.

3.3. Comparison of the Relationship between  $\tau_c$  and  $\tau_o$ , and Their Influencing Factors

The  $\tau_c$  was independent of  $S$  in the WEPP model, and thus, only the influence of  $S$  on  $\tau_o$  was evaluated. As shown in Figure 4, a significant positive relationship existed between  $\tau_o$  and  $S$ , with  $R^2$  ranging from 0.76 to 0.99 and NSE varying from 0.74 to 0.98. All sampling sites showed power functions between  $\tau_o$  and  $S$ .

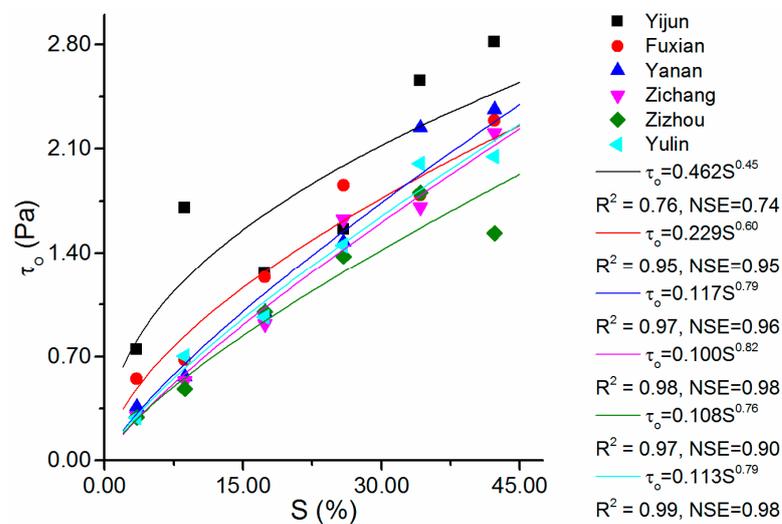


Figure 4. The relationship between  $\tau_o$  and slope gradient ( $S$ ).

Regarding the impact of soil properties on  $\tau_o$  and  $\tau_c$ , no significant relationship was found between  $\tau_c$  and the measured soil properties. Similarly, no relationships existed among clay content, silt content, and sand content and  $\tau_o$ . An exponential function of  $\tau_o$  and SOM was expressed ( $R^2 = 0.64$ ,  $NSE = 0.53$ ; Figure 5).

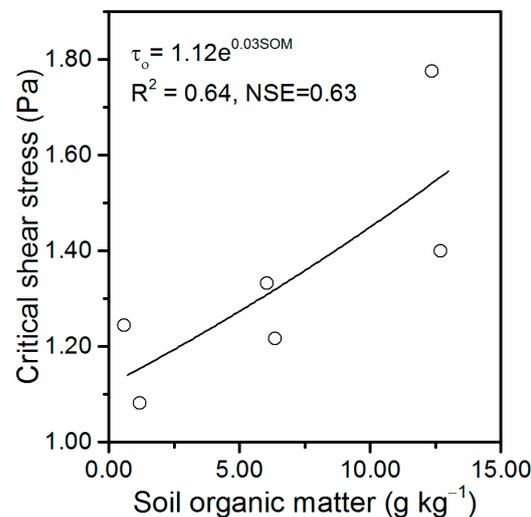


Figure 5. The relationship between  $\tau_o$  and soil organic matter (SOM).

#### 4. Discussion

Nearing et al. [12] defined  $\tau_c$  as the threshold of rill detachment below in which soil detachment is zero. In this study, nearly all sampling sites had obviously greater  $\tau_c$  than  $\tau_o$ . Interestingly, an obvious soil detachment was apparent for sampling sites with higher  $\tau_c$ , which might contradict the definition of  $\tau_c$  [12]. This result meant that  $\tau_c$  calculated by Equation (3) could not represent the critical condition of the start of soil detachment. The definition of  $\tau_c$  has been contested [33,56]. For example, Knapen et al. [46] and Wang et al. [57] have stated that  $\tau_c$  represented the appearance of significant detachment, which still did not explain why significant detachment was observed at the sampling sites with higher  $\tau_c$ . Therefore, the reliability of  $\tau_c$  determined by Equation (3) was probably controversial. Nearing et al. [32] also indicated  $\tau_c$  had no actual meaning, as it was calculated by linear regression, and a large regression slope would lead to a large value ( $\tau_c$ ).

The gradually increasing trend of  $K_r$  could be well explained by the soil properties. Specifically,  $K_r$  was negatively correlated to clay content and silt content, and a positive relationship was observed between sand content and  $K_r$  (Table 6). This finding was consistent with the results of Geng et al. [21]. Some previous studies have shown  $K_r$  decreased with  $\tau_c$ , implying that factors that influenced  $K_r$  also affected  $\tau_c$  [58]. Although  $K_r$  showed an increasing trend from south to north and had a satisfactory relationship with soil properties, no significant relationship was observed between  $K_r$  and  $\tau_c$ . Furthermore, Knapen et al. [59] indicated that the  $K_r$  of deep noninversion tillage was 10 times lower than that of conventional plowing, but the differences in  $\tau_c$  between these two tillage practices were not significant. Additionally, Geng et al. [21] calculated  $K_r$  and  $\tau_c$  of 36 soil types across eastern China and also found no significant relationship between  $K_r$  and  $\tau_c$ . For the relationship between  $K_r$  and  $\tau_o$ ,  $K_r$  increased with an increase of  $\tau_c$  from Yijun to Zizhou, which supported the perception that the higher  $\tau_o$  corresponded to the lower  $K_r$ . The increasing trends of  $\tau_o$  and  $K_r$  from Zizhou to Yunlin barely supported the inverse relationship between  $\tau_o$  and  $K_r$ , which could be attributed to the low clay content and high sand content in Yulin (Table 5). Sand particles need higher flow shear stress to be detached because of their high ratio of mass to surface area [60]. Although Yulin has high  $\tau_o$ , Yulin also showed high  $K_r$ . The low clay content of this sampling site may explain both the high  $K_r$  and  $\tau_o$  of Yulin [21].

For  $\tau_c$ , the widely applied WEPP model considered that  $\tau_c$  did not change with the variation of  $S$  [14,25,34]. This study, however, found a significantly positive relationship between  $\tau_o$  and  $S$ . This result was the opposite of the common-sense understanding that the gravitational force along a flume bed increases with  $S$ , which makes soil particles detach more easily [31]. Lei and Nearing [34] also found an increase of  $\tau_o$  with  $S$  for loose material. In this study, the ratios of the  $H_o$  ( $S$ , 42.26%) to the  $H_o$  ( $S$ , 3.49%) for Yijun, Fuxian, Yanan, Zichang, Zizhou, and Yulin were 31.08%, 34.30%, 54.12%, 61.04%, 58.14%, and 58.37%, respectively. Nonetheless, the  $S$  (42.26%) was 12.11 times that of the  $S$  (3.49%). Thus, the influence of  $S$  on  $\tau_o$  was much greater than  $H_o$  and explained why  $\tau_o$  increased with  $S$  [34]. This statement was inconsistent with the findings of some researchers who found that the critical shear stress was related only to soil properties [25,28]. Nonetheless, hydraulic parameters and  $S$  also have been used to calculate the critical shear stress [24,61]. This discrepancy has probably reflected the lack of clarity regarding the significance of the critical shear stress [7].

Numerous disputes have existed with regard to the influence of soil properties on  $\tau_c$  [21,24]. This study demonstrated a poor relationship between  $\tau_c$  and measured soil properties. Our results, however, agreed with the findings of Geng et al. [21]. They explored the relationship between  $\tau_c$  and 17 soil properties of 36 soil types and also did not find a significant relationship between  $\tau_c$  and the 17 soil properties. For the relationship between  $\tau_o$  and measured soil properties,  $\tau_o$  increased exponentially and SOM (Figure 5), which agreed with the findings of Kimiaghali et al. [62] and Grabowski et al. [63]. The poor relationship between  $\tau_o$  and soil texture was probably caused by the fact that both the

cohesive effect of clay particles and the ratio of mass to surface area of sand particles determined  $\tau_o$ , and their interaction was complicated [59].

Generally speaking,  $\tau_o$ , compared with  $\tau_c$ , did not present significantly better results with regard to the relationships with  $K_r$ ,  $S$ , and soil properties. This finding could deepen the recognition of erosion processes and improve the precision of process-based soil erosion models. In this study, however, only the relationships between critical shear stress and some basic soil properties (soil texture and SOM) were established and compared, and the relationships with other key soil properties (such as soil cohesion, aggregate stability, plant root, etc.) were not investigated [64]. Therefore, further research should be conducted to compare the relationships between  $\tau_o$  and  $\tau_c$  and these influencing factors. Even though  $\tau_o$  was more reliable compared to  $\tau_c$ , observing the start of soil particle detachment was subjective [54]. Thus, it was meaningful to develop an objective method to ascertain the beginning of soil particle detachment [39,65]. Additionally, some researchers have argued that the critical shear stress had a relationship only with soil properties, which as well revealed the uncertainty of using a method to determine  $\tau_o$  that was related to hydraulic parameters and  $S$  [28].

## 5. Conclusions

This study aimed to identify the reliability of critical shear stress in rills by observing the start of soil particle detachment.  $\tau_c$  was determined by linear regression, and  $\tau_o$  was measured by observing the soil particle detachment of six sampling sites along a transect of the Loess Plateau. The results showed that the mean  $\tau_c$  (2.55 Pa) was 1.90 times that of  $\tau_o$  (1.34 Pa).  $\tau_c$  changed irregularly from south to north along the transect, and  $\tau_o$  decreased from Yijun to Zizhou and then increased from Zizhou to Yulin. No significant relationship was observed between  $\tau_c$  and rill erodibility ( $K_r$ ).  $K_r$  increased with a decrease of  $\tau_o$  from Yijun to Zizhou, whereas both  $K_r$  and  $\tau_o$  followed increasing trends from Zizhou to Yulin. Although  $\tau_c$  was deemed unchanged with  $S$ ,  $\tau_o$  followed an obviously increasing trend with the increase of  $S$  with great  $R^2$  (0.76 to 0.99) and NSE (0.74 to 0.98). No significant relationship existed between  $\tau_c$  and measured soil properties ( $p > 0.05$ ); however,  $\tau_o$  exhibited exponentially increasing functions with SOM ( $R^2 = 0.64$ ; NSE = 0.63). Overall,  $\tau_o$  displayed no significantly better results compared with  $\tau_c$ , considering the relationships with  $K_r$ ,  $S$ , and soil properties. Additionally, some uncertainties also remained regarding the method used to determine  $\tau_o$ . The results of this study were helpful for the accurate measurement of critical shear stress in rills and for improving the prediction accuracy of the process-based soil erosion model.

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